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UNITED STATES
DEPARTMENT OF THE INTERIOR
BUREAU OF RECLAMATION



HYDRAULIC MODEL STUDY OF GALINDO CREEK SIDE CHANNEL WASTEWAY

CONTRA COSTA CANAL
CENTRAL VALLEY PROJECT, CALIFORNIA

Hydraulic Laboratory Report No. Hyd-213



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BRANCH OF DESIGN AND CONSTRUCTION DENVER, COLORADO

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Hydraulic Laboratory
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Subject: Hydraulic model study of Galindo Creek sidechannel wasteway-Contra Costa Canal--Central Valley Project, California.

INTROLUCTION

Hydraulic model studies of the Galinoo Creek sidechannel wasteway of the Contra Costa Canal in the Central Valley Project of California were performed in the Hydraulic Laboratory in Denver in September 1939. The report was not completed at that time due to more urgent work in the laboratory.

The prototype sidechannel wasteway, Figure 1, starts at canal station 1726/67.42 and is 25 feet long on an 80-foot radius horizontal curve. The purpose of the wasteway is to prevent the overtopping of the canal banks. The sidechannel wasteway discharges into a tunnel and finally into a stilling-pool before entering into the wasteway canal. The principle purpose of the model study was to determine whether the capacity of the sidechannel wasteway was adequate.

THE NOLEL

A 1:6 scale model of the sidechannel wasteway was constructed and located in a model channel previously used to determine losses for flow around piers in wash overchutes for the Coachella Canal. The sideslopes of the model canal were 1-1/2:1, whereas those of the Contra Costa Canal are 1-1/4:1. The discrepancy was thought to be insignificant in regard to the general operation of the sidechannel. It was also considered unnecessary to reproduce the slight channel curvature to determine the approximate dimensions of the sidechannel.

OPERATION OF THE ORIGINAL DESIGN

Operation of the original design showed the sidechannel to be insdequate. The spillway crest was nearly 100 percent submerged over its entire length (Figures 2 and 3). The head on the crest was 0.486 feet prototype, making the upstream canal water surface elevation 101.236. With the top of the canal lining at elevation 101.25, there was practically no freeboard.

OPERATION OF DESIGN B

To prevent submergence of the wasteway crest, the bottom sidechannel was lowered 6 inches prototype. For this case there was practically no submergence (Design B, Figures 2 and 3). The discharge coefficient was increased from 2.85 to 3.48 for the maximum discharge conditions, and the upstream canal water surface was lowered to elevation 101.175.

OPERATION OF DESIGN C

Lowering the bottom of the sidechannel 4 inches more eliminated all submergence but the discharge coefficient was increased only slightly to 3.53 with the canal water surface at elevation 101.172. The water surfaces in the sidechannel for this condition are shown as Design C, Figures 2 and 3.

CONCLUSIONS AND RECOMMENDATIONS

Due to the small hydraulic improvement encountered in the latter

Design C and the satisfactory operation of the sidechannel for the

intermediate floor position, Design B with the sidechannel floor located

6 inches below the original design is recommended. The recommended design
is shown as Figure 1.

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